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WAVE FORCES ON A LARGE STRUCTURE IN THE PRESENCE OF A CURRENT

Sung-Shan Hsiao* and Ming-Chung Lin**

Keywords: Wave forces, Large Structure, Wave-current Coexisting Field.

ABSTRACT

The wave forces acting on a large structure in the presence of a current have been investigated numerically. It is noted that the velocity potential in a wave-current coexisting field can be separated into an unsteady wave potential and a steady current potential. The unsteady wave potential with effects of both a uniform current and a large structure was calculated by using boundary element method. The steady current potential can be expressed as the sum of a uniform current and a steady disturbance due to the existence of the structure. This means that current velocity will be changed in the vicinity of the structure. The variation of current velocity around the structure was then calculated by using a surface vorticity method. Substituting both unsteady wave potential and current velocity distribution into the first-order dynamic pressure equation and integrating the pressure over the body surface, the first-order wave forces on a large structure in the presence of a current then can be obtained. Comparison of the present results with those of Matsui *et al.* (1991) are made. The agreement is satisfactory. The numerical model proposed here has its adaptability to the problem of waves, current and a large structure interactions.

INTRODUCTION

When waves propagate toward a large structure, the existence of currents in ocean having different characteristics complicates the problem concerning the interactions between wave and current around the structure. With increasing engineering activities in both coastal and offshore regions nowadays, accurate predictions of wave forces and, thereby the responses of large structures, in the presence of currents have become progressively important.

To study the interactions between current, wave

and slender cylinders, Iwagaki and Asano (1984) performed flow visualization tests to examine the vortex property around a circular cylinder in a wave-adverse current coexisting field. The hydrodynamic forces acting on the cylinder are also discussed by them. Based on the linear wave theory, Li and Zhang (1990) investigated forces on cylinders under the actions of both regular waves and currents. A series model tests of regular and irregular wave-current forces on slender circular cylinders were carried out by Li *et al.* (1991).

To investigate the interactions of wave-current around a large structure, Zhao and Faltinsen (1988) developed a theory to the first-order in wave amplitude and current velocity. An algorithm to solve the interaction between waves and currents with a two-dimensional body was also presented by them. Lin and Hsu (1991) proposed a numerical scheme to study the interactions of wave-current around a large structure. The numerical scheme is based on the mild-slope wave equation derived by Kirby (1984), where the effects of wave refraction-diffraction, together with currents, on a slowly varying topography were considered. To verify the validity of their numerical scheme, model tests in a laboratory basin were also conducted by Lin *et al.* (1990). The hydrodynamic forces on a vertical cylinder in current and waves were also analyzed by Matsui *et al.* (1991). The significant influence on hydrodynamic forces of the cylinder due to the presence of a current is reported. You and Tao (1992) investigated the nonlinear wave-current forces on large scale cylinders by time-stepping method. Isaacson and Cheung (1993) presented a time-domain solution for wave-current interactions with a two-dimensional body. A numerical model using finite/boundary element method was proposed by Lin *et al.* (1994) to deal with wave refraction-diffraction in the presence of a current. Lin and Hsiao (1994) proposed a numerical approach for wave-current interaction around a large structure. The distribution of wave height around a large cylinder in a wave-current coexisting field was calculated and compared with experimental results. Wave height variations around a large cylinder due to wave-current

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interactions were studied experimentally by Lin and Hsiao (1995). Both the cases of slowly varying topography, as well as with constant water depth, were considered.

The general approach for the velocity potential of a wave-current coexisting field is to separate the velocity potential into a steady current potential and an unsteady wave potential. The steady current potential can be expressed as the sum of a uniform current and a steady disturbance which resulted through the presence of the structure in the flow field. This then leads to alteration of the current velocity in the vicinity of the structure. For a structure of arbitrary shape, it is difficult to find a closed form solution but a numerical solution may be obtained by a direct application of an integral equation method (Isaacson and Cheung, 1993). In this paper, the surface vorticity method due to Lewis (1991) was used to obtain the needed boundary integral equation. The variation of the current velocity due to the existence of the structure was then directly found by solving this equation numerically. Boundary element method was used to calculate the unsteady wave potential which resulted from the presence of a uniform current and the structure. Assuming that the wave is

small amplitude motion and the current velocity is rather small, one can obtain a first-order dynamic pressure equation. Substituting both the unsteady wave potential and the current velocity into the equation and integrating the pressure over the structure surface, the first-order wave forces on a large structure in the presence of a current were obtained.

THEORETICAL FORMULATION

The problem of wave forces acting on a large structure under the effect of a current is considered. With reference to figure 1, the problem is defined with a fixed coordinate system o - xyz , in which xy -plane is on the still water level, o is at the center of the body, and z is measured vertically upwards from the xy -plane. The seabed is assumed to be impermeable and horizontal along the plane $z = -h$. U is the current velocity and is assumed to be uniform with depth, k is the wave number of incident waves. α and β are, respectively, the inclinations of wave and current to the x axis, $\theta = \beta - \alpha$ is then the angle between waves and current.

1. Velocity potential

Assuming fluid in the wave-current coexisting field is incompressible and inviscid, and the flow irrotational, the fluid motion can then be described by a velocity potential Φ which satisfies the Laplace equation:

$$\nabla^2 \Phi = 0, \quad (1)$$

The potential Φ is also subjected to boundary conditions on the seabed, the structure surface, and the free surface, which are given, respectively, as

$$\frac{\partial \Phi}{\partial z} = 0, \quad z = -h \quad (2)$$

$$\frac{\partial \Phi}{\partial n} = 0, \quad \text{on } S_1 \quad (3)$$

$$\frac{\partial \Phi}{\partial t} + \frac{1}{2}(\nabla \Phi \cdot \nabla \Phi) + gz = c, \quad z = \eta \quad (4)$$

Here, $\frac{\partial}{\partial n}$ is the derivative, normal to S_1 , the surface of the structure; t denotes time and g is the gravitational constant; η and c are, respectively, the water surface elevation above the still water level and the Bernoulli's constant.

The potential Φ can be separated into a steady current potential and an unsteady wave potential, as

$$\Phi = \phi_u + \phi_w \quad (5)$$

The current velocity in the vicinity of the structure will be altered due to the presence of the object. Thus,

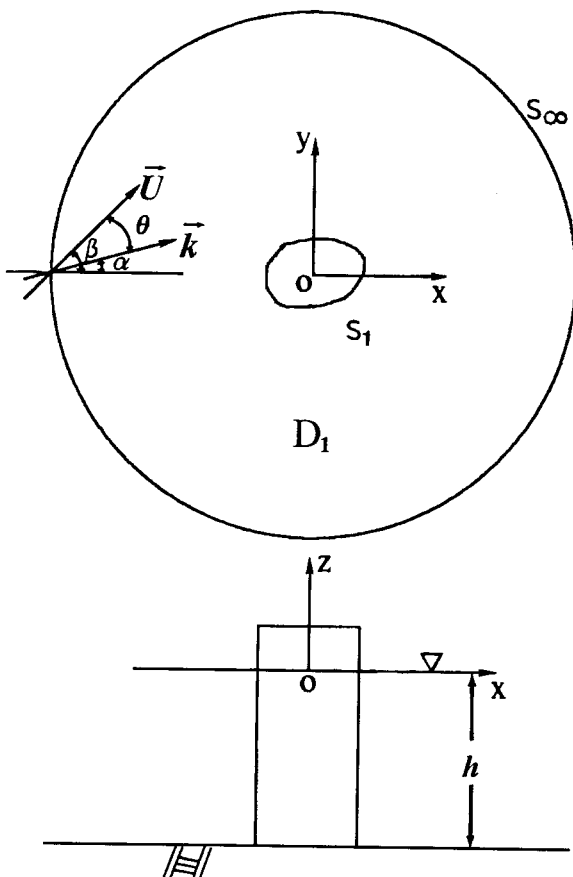


Fig. 1. Definition sketch of the computational domain.

the steady current potential ϕ_u can be expressed more conveniently as the sum of a uniform current potential $\vec{U} \cdot \vec{x}$ and a steady disturbance potential ϕ_b caused by the structure in the flow (Isaacson and Cheung, 1993).

$$\phi_u = \vec{U} \cdot \vec{x} + \phi_b \quad (6)$$

The unsteady wave potential ϕ_w in a wave-current coexisting field is given by linear wave theory, as

$$\phi_w = -\frac{igA}{\sigma} \frac{\cosh k(z+h)}{\cosh kh} f e^{-i\omega t} \quad (7)$$

where A is amplitude of the incident wave, i is the imaginary constant, and g is the gravitational constant, ω and σ , denote, respectively, the absolute and relative angular frequency in the wave-current coexisting field. The wave number k and the angular frequency ω , σ are related by the linear dispersion and Doppler relation such as

$$(\omega - \vec{k} \cdot \vec{U})^2 = gk \tanh kh \quad (8)$$

$$\sigma = \omega - \vec{k} \cdot \vec{U} \quad (9)$$

In equation (7), f denotes the surface potential function. It is noted that, using the diffraction theory approach, f can be expressed as a linear combination of surface potential functions resulting from both incident and scattered waves:

$$f = f^i + f^s \quad (10)$$

where f^i is surface potential function of the incident wave, and can be expressed as

$$f^i = \exp[ik(x \cos \alpha + y \sin \alpha)] \quad (11)$$

Substituting equations (5), (6), (7), (10) and (11) into equation (1), and applying boundary conditions on the surface of the structure, as well as a far-field radiation boundary condition, the surface potential function of the scattered wave f^s in the analytical domain satisfying Helmholtz equation is given by:

$$\nabla_h^2 f^s + k^2 f^s = 0, \quad \nabla_h = (\partial/\partial x, \partial/\partial y) \quad (12)$$

$$\frac{\partial f^s}{\partial n} = -\frac{\partial f^i}{\partial n}, \quad \text{on } S_1 \quad (13)$$

$$\lim_{r \rightarrow \infty} \sqrt{r} \left(\frac{\partial}{\partial r} - ik \right) f^s = 0, \quad \text{on } S_\infty \quad (14)$$

Equations (13) and (14) are boundary conditions on the surface of the structure, and the far-field radiation boundary condition, respectively.

2. Wave forces

The wave forces acting on a body can be determined by carried out appropriate integration of the pressure over the body surface. In the present application, the pressure in the fluid may be determined by the Bernoulli equation :

$$p = -\rho \left[\frac{\partial \Phi}{\partial t} + \frac{1}{2} (\nabla \Phi \cdot \nabla \Phi) + gz \right] \quad (15)$$

where ρ is the fluid density, and t denotes time. Substituting equation (5) into equation (15), and without considering the hydrostatic pressure term ρgz , one can obtain a dynamic pressure equation:

$$p = -\rho \left[\frac{\partial \phi_w}{\partial t} + \nabla \phi_u \cdot \nabla \phi_w + \frac{1}{2} (|\nabla \phi_u|^2 + |\nabla \phi_w|^2) \right] \quad (16)$$

Considering the case of small amplitude wave motion and small current velocity in the wave-current coexisting field, equation (16) can be rewritten into a first-order dynamic pressure equation expressed as

$$p = -\rho \left[\frac{\partial \phi_w}{\partial t} + \nabla \phi_u \cdot \nabla \phi_w \right] \quad (17)$$

It should be noted that the term $\nabla \phi_u \cdot \nabla \phi_w$ is of first order in wave amplitude, and accounts for the interaction between waves and current. Substituting equation (7) into equation (17), the first-order dynamic pressure equation is rewritten as:

$$p = \frac{\rho g A}{\sigma} [\omega f + i(\nabla \phi_u \cdot \nabla f)] \frac{\cosh k(z+h)}{\cosh kh} e^{-i\omega t} \quad (18)$$

The first-order wave forces acting on the body under the effect of a current can be determined by the integration of the first-order dynamic pressure equation over the surface of the object and the water depth. i.e.

$$\left. \begin{aligned} F_x &= \int_{-h}^0 \int_{S_1} p dy dz \\ F_y &= \int_{-h}^0 \int_{S_1} p dx dz \end{aligned} \right\} \quad (19)$$

where F_x , F_y represent, respectively, the first-order force components in the x and y directions.

NUMERICAL PROCEDURE

1. Calculation of current velocity

Consider the case of a flow past a two-dimensional body lying in the xy -plane. The object is assumed to be immersed in a uniform current which is inclined at an angle β to the x axis (Fig. 2). Applying the surface

vorticity method (Lewis, 1991), one can represent this flow by a distributed vorticity sheet clothing the whole body but with initially unknown strength. Making use of the Biot-Savart law and boundary condition on the surface at m , a boundary integral equation can be obtained.

$$\oint \gamma(s_n)K(s_m, s_n)ds_n - \frac{1}{2} \gamma(s_m) = -U \left(\frac{dx_m}{ds_m} \cos \beta + \frac{dy_m}{ds_m} \sin \beta \right) \quad (20)$$

where the last term is the component of U parallel to the body surface. $K(s_m, s_n)$ is a coupling coefficient whose value is that of the velocity at parallel to the body surface, induced by a unit vorticity located at s_n , namely

$$K(s_m, s_n) = \frac{1}{2\pi} \left\{ \frac{-(x_m - x_n)(dy_m / ds_m) + (y_m - y_n)(dx_m / ds_m)}{(x_m - x_n)^2 + (y_m - y_n)^2} \right\} \quad (21)$$

Equation (20) is to be satisfied at all points on the body surface. A practical approach to approximate this would be to divide the body surface into N discrete elements Δs_n , and pivotal points are selected at the mid-points of the elements (Fig. 3), whereupon, equation (20) may be expressed as a linear equation of the form:

$$\sum_{n=1}^N \gamma(s_n)K(m,n)\Delta s_n = -U \left(\frac{dx_m}{ds_m} \cos \beta + \frac{dy_m}{ds_m} \sin \beta \right) \quad (22)$$

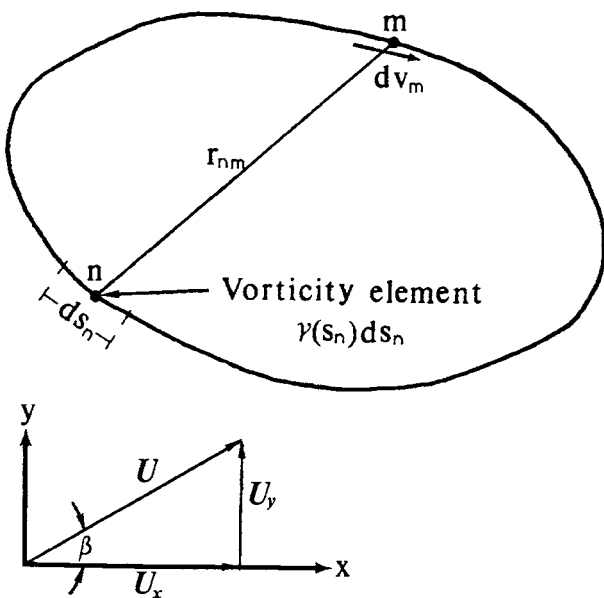


Fig. 2. Surface vorticity model for potential flow past a two-dimensional body.

Here, for simplicity, the $\gamma(s_m)/2$ term on equation (20) has been absorbed into coupling coefficient $K(m,n)$, thus

$$K(m,n) = K(s_m, s_m) - \frac{1}{2}; n=m \\ = K(s_m, s_n); n \neq m \quad (23)$$

Lewis (1991) showed that the coupling coefficient $K(m, n)$ acting upon itself is given by

$$K(m,m) = \Delta\theta_m / 4\pi \quad (24)$$

where $\Delta\theta_m$ is the angle subtended by the curved element Δs_m (Fig. 3), and may be approximated by half the change of the slope between adjacent elements, namely

$$\Delta\theta_m = \frac{1}{2} \left(\frac{dy_{m-1}}{dx_{m-1}} - \frac{dy_{m+1}}{dx_{m+1}} \right) \quad (25)$$

A suitable discretization is achieved such that equation (22) must be written for the pivotal point of each element, resulting in a set of N equations for N unknown values of surface vorticity, $\gamma(1), \gamma(2), \dots, \gamma(N)$. Equation (22) then can be written in the matrix form:

$$\begin{bmatrix} K_{11} & \dots & \dots & K_{1N} \\ K_{21} & \dots & \dots & K_{2N} \\ \vdots & \vdots & \vdots & \vdots \\ K_{N1} & \dots & \dots & K_{NN} \end{bmatrix} \begin{bmatrix} \gamma(s_1) \\ \gamma(s_2) \\ \vdots \\ \gamma(s_N) \end{bmatrix} = \begin{bmatrix} rhs_1 \\ rhs_2 \\ \vdots \\ rhs_N \end{bmatrix} \quad (26)$$

with simplified notation $K_{mn} = K(s_m, s_n)$ and the right hand sides given by

$$rhs_m = -U \left[\frac{dx_m}{ds_m} \cos \beta + \frac{dy_m}{ds_m} \sin \beta \right] \quad (27)$$

Solution of the above N linear equations then yield directly the value of surface vorticity $\gamma(s_m)$. The current velocity close to the body surface ($\nabla\phi_u$) _{m} is now immediately known, being equal to the surface vorticity $\gamma(s_m)$ (Lewis, 1991).

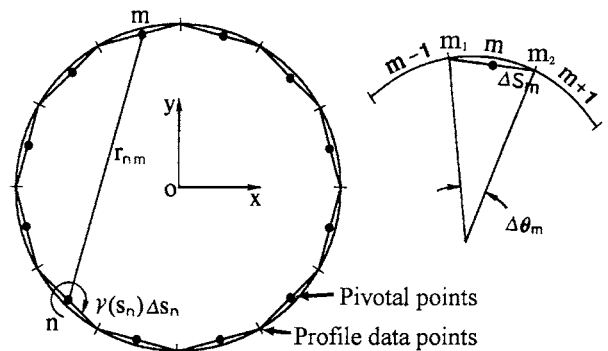


Fig. 3. Boundary element geometry.

2. Calculation of wave potential function

Application of Green's second identity, and with the boundary condition on (equation (14)) to equation (12) leads to the following boundary integral equation :

$$C_{if_i^s} = \int_{S_1} (f_j^s \frac{\partial f^*}{\partial n} - f^* \frac{\partial f_j^s}{\partial n}) ds$$

$$C_i = \begin{cases} 1 & \text{in the analytical domain } D_1 \\ \frac{1}{2} & \text{on the boundary } S_1 \end{cases} \quad (28)$$

where $\partial/\partial n$ is the normal derivative, directed outward from the fluid region at the structure surface. f^* is the fundamental solution of the Helmholtz equation. Expressing the fundamental solution in terms of the Hankel function of the first kind of zero order, one has

$$f^* = -\frac{i}{4} H_0^{(1)}(kr) \quad (29)$$

where r is the distance from the source point to the point under consideration. For the discretization of equation (27), constant elements on the boundary are used. Equation (28) can then be approximated and rewritten as

$$\sum_{j=1}^N f_j^s \int_{s_j} \frac{\partial f^*}{\partial n} ds - C_{if_i^s} = \sum_{j=1}^N \frac{\partial f_j^*}{\partial n} \int_{s_j} f^* ds \quad (30)$$

This equation is to be satisfied at all points of the surface S_1 . The resulting simultaneous equations written in matrix form can be expressed as

$$[H_{ij}][F_j] = [G_{ij}][F_j^*] \quad (31)$$

where

$$H_{ij} = \int_{s_j} \frac{\partial}{\partial n} (-\frac{i}{4} H_0^{(1)}(kr)) ds \quad (32)$$

$$G_{ij} = \int_{s_j} (-\frac{i}{4} H_0^{(1)}(kr)) ds \quad (33)$$

$$F_j^* = \frac{\partial f_j^s}{\partial n} = -\frac{\partial f_j^i}{\partial n} \quad (34)$$

From equation (31), the unknown quantities of the scattered wave potential function $[F_j]$ on boundary S_1 are solved by matrix inversion. Then the surface wave potential function $f = f^i + f^s$ is obtained.

RESULTS AND DISCUSSION

To examine and illustrate the present method, the special case of a large circular cylinder with radius a in constant water depth (h) is considered. The incident

waves are progressive in the positive x direction. For numerical computation, the ratio of water depth to the radius of the cylinder $h/a = 1.0$, the Froude number $F_r = U/\sqrt{g\bar{a}} = 0.05, 0.10$, and different angles ($\theta = 0^\circ, 45^\circ, 90^\circ, 135^\circ, 180^\circ$) between waves and current are chosen respectively. The wave number k of a linear wave interacting with the current U is obtained from the linear dispersion relation (equation (8)) with the value of ω ($\omega^2 = gk' \tanh k'h$, where k' is the zero-current wave number) (Thomas, 1981). With the value of ka , the first-order wave forces can be obtained from equation (19). Plotting them against ka , the variation of wave forces under the effect of a current can be seen clearly. For a large structure discussed in this study, the value of $2a/L$ (L is the incident waves length) must be large than 0.2 (Brebbia, 1979). Therefore, the region of $ka \leq 0.2\pi$ can be out of consideration.

To verify the validity of this numerical model, wave forces on the cylinder without current effect (i.e. $F_r = 0$) were calculated firstly. Figure 4 shows the calculated results together with the analytical solutions of MacCamy and Fuchs (1954). The agreement is rather satisfactory. A comparison is also made of the present numerical results for $F_r = 0.10$ with those of Matsui *et al.* (1991) in figure 5. It indicates that the present results are in good agreement with those of Matsui *et al.* (1991) except for $ka = 1.5 \sim 2.0$.

Variation of wave forces under the effects of following and opposing current with parameter ka is presented in figure 6. It demonstrates that wave forces increase with increasing value of Froude number in the small wave number region ($ka < 1.0$), but decrease with increasing value of Froude number in the large wave number region ($ka > 1.0$). Wave forces are larger than

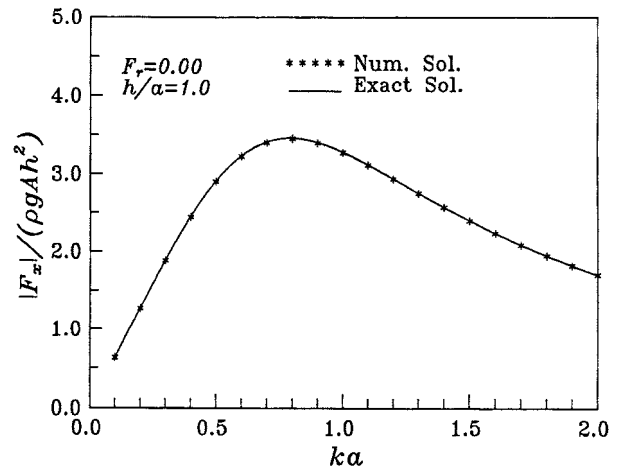


Fig. 4. Comparison of wave forces in absence of current between the numerical and exact solution.

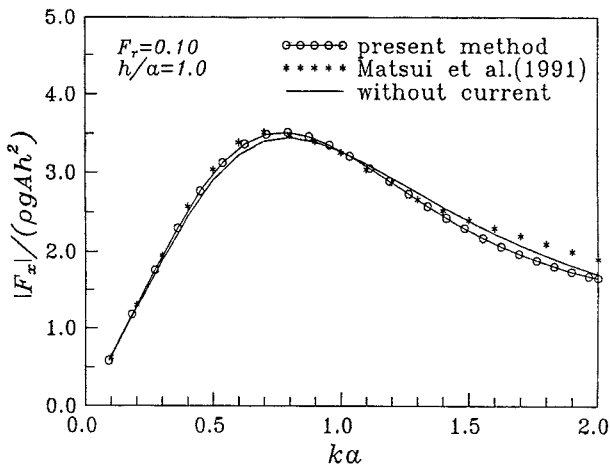


Fig. 5. Comparison of present results with those of Matsui et al. (1991) for $F_r = 0.10$.

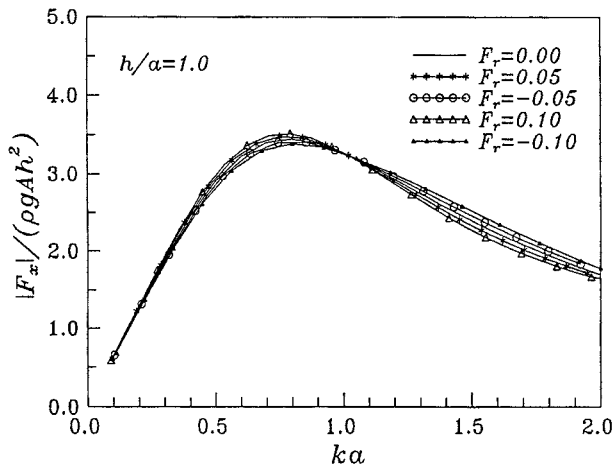


Fig. 6. Variation of wave forces with ka under the effects of both following and opposing current.

that of no current present in the small wave number region for $F_r > 0$ (i.e. under the effect of a following current), but smaller for Froude number $F_r < 0$ (i.e. under the effect of an opposing current). An opposite trend can be observed in the large wave number region. This phenomenon can be due to the fact that the curve of the wave forces with no current present has a positive slope in the small wave number region, but has a negative slope in the large wave number region. For the reason of increasing wave length under the effect of a following current, i.e. the value of ka is smaller than the relative diffraction parameter $k'a$ of zero-current, so that the wave forces larger than that of current-free case in the small wave number region, but smaller in the large wave number region. The opposite trend is true for the case of an opposing current.

When the angles (θ) between waves and current

are 45° and 135° , waves will be refracted due to the influence of current. Therefore, the wave forces acting on the cylinder under the effect of a current have force components in the y -direction, besides in the x -direction. Figures 7 and 8 show the relationship between wave forces component in the x -direction and the diffraction parameter ka . From these figures, it can be seen that, (a) with waves propagating on a favorable current ($\theta < 90^\circ$), wave forces in the x -direction become smaller compare to that of no current present in the large wave number region, and also decrease as the value of F_r increases, (b) with waves propagating on an adverse current ($\theta > 90^\circ$), an opposite trend is found. Figure 9 demonstrates that wave force components in the y -direction increases as the value of F_r increases. It means that the wave force components in the y -direction increases with increasing velocity of current.

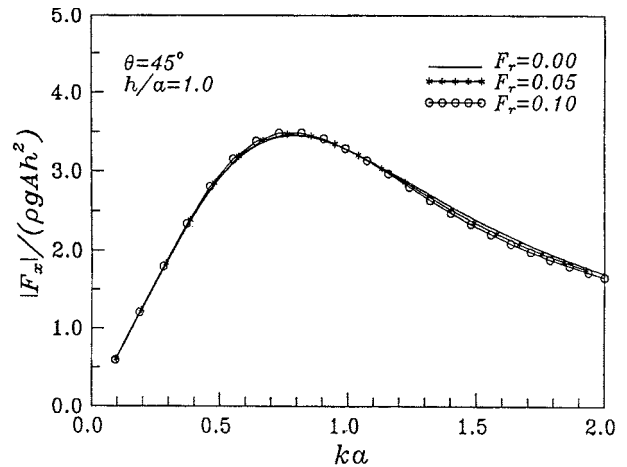


Fig. 7. Variation of wave forces in the x -direction with ka for $\theta = 45^\circ$ and different Froude numbers.

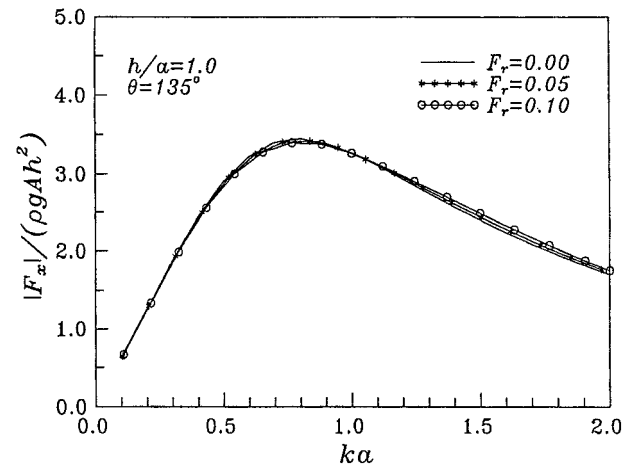


Fig. 8. Variation of wave forces in the x -direction with ka for $\theta = 135^\circ$ and different Froude numbers.

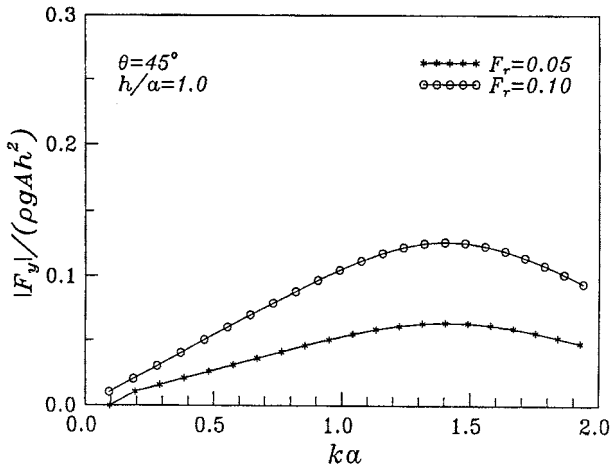


Fig. 9. Variation of wave forces in the y-direction with ka for $\theta = 45^\circ$ and different Froude numbers.

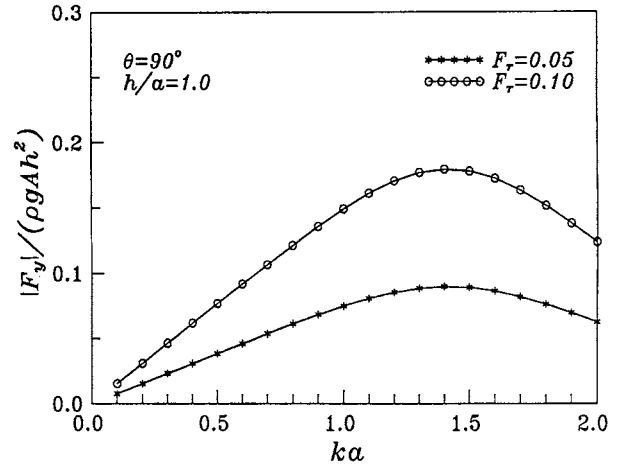


Fig. 11. Variation of wave forces in the y-direction with ka for $\theta = 90^\circ$ and different Froude numbers.

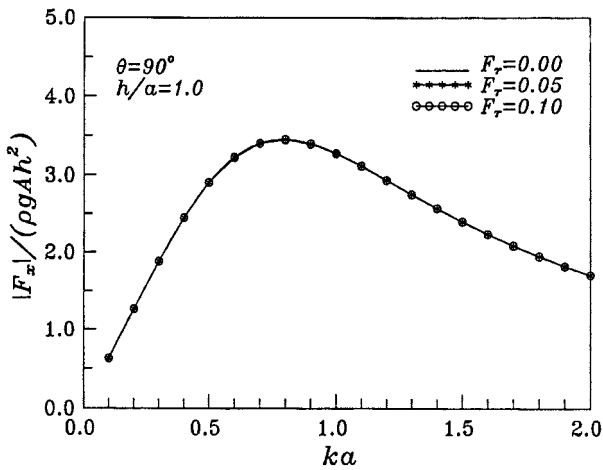


Fig. 10. Variation of wave forces in the x-direction with ka for $\theta = 90^\circ$ and different Froude numbers.

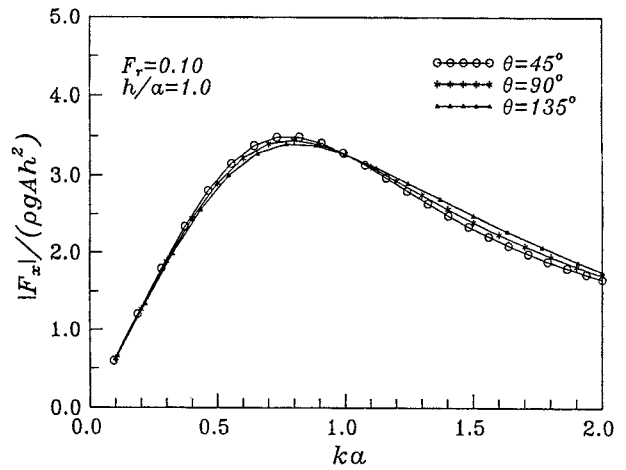


Fig. 12. Variation of wave forces in the x-direction with ka for $F_r = 0.10$ and different angles between waves and current.

When the angle between waves and current is equal to 90° , the velocity of current has no component in the direction of the incident wave, so that the wave length will not be affected by current. Therefore, no matter what the value of F_r is, the wave forces in the x-direction are the same as no current present (Fig. 10). Nevertheless, the wave force components in the y-direction are still affected by the existence of a current, and increase as the velocity of current increases (Fig. 11).

Figures 12 and 13 show the plots of wave forces in the x and y-directions respectively, for different angles between waves and current, but with the same value of F_r (i.e. the same current magnitude). Figure 12 reveals that the wave forces in the x-direction decrease with increasing angle between waves and current in the small

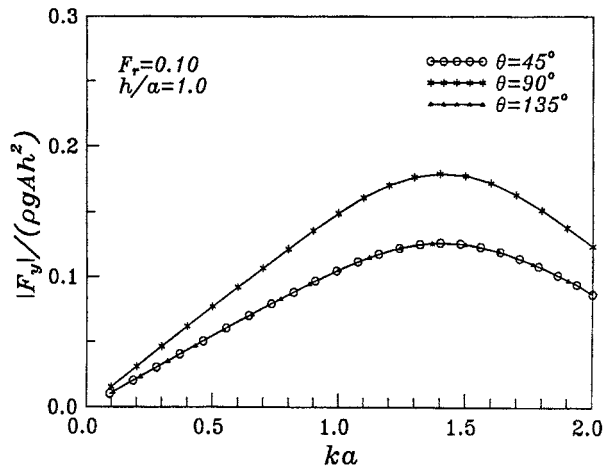


Fig. 13. Variation of wave forces in the y-direction with ka for $F_r = 0.10$ and different angles between waves and current.

wave number region, but increase as the angle (θ) increases in the large wave number region. This is the fact that waves travel with current for $\theta < 90^\circ$, whereas, it travels against the current for $\theta > 90^\circ$. It can be therefore concluded that, the wave forces for the case of a following current is smaller than that without current in the large wave number region, but with an opposing current, a completely opposite trend can be detected. In Figure 13, it is found that the wave forces in the y -direction have a maximum value for $\theta = 90^\circ$, and have the same value for $\theta = 45^\circ$ and $\theta = 135^\circ$. The reason for this is because that the velocity component of current in the y -direction has a maximum value when $\theta = 90^\circ$, and has the same value for $\theta = 45^\circ$ and $\theta = 135^\circ$.

CONCLUSIONS

In this study, the surface vorticity method and boundary element method are combined to investigate the first-order wave forces on a large structure in the presence of a current. For the validity of this numerical model, comparison of the present results with the analytical solutions of MacCamy and Fuchs (1954) for zero-current, and with those of Matsui *et al.* (1991) for $F_r = 0.10$ are made. The agreement is satisfactory. The numerical model proposed here has its adaptability to the problem of waves, current and a large structure interactions.

The following conclusions can be drawn from the numerical results :

1. Wave forces on a large body are larger than that of no current present in small wave number region for the incident waves travel with current, but smaller for the incident waves travel against current, and also increase with increasing value of Froude number. An opposite trend can be observed in the large wave number region.
2. When the intersection angle between waves and current is not equal to 0° or 180° , it exists wave forces component in the y -direction. The wave forces increase as the velocity of current increases.
3. When the angle between waves and current is equal to 90° , the wave length will not be affected by current, the wave forces component on the x -direction are the same as that with no current present, nevertheless the wave forces component in the y -direction are still affected by the existence of a current, and increase as increasing current magnitude.
4. For a fixed current velocity (the value of Froude number is constant), but with different angles between waves and current, the wave forces in the x -direction increases as the angle increases in the large wave number region. An opposite trend can be observed in the small wave number region. Whereas

the wave forces in the y -direction increase as the angle increases until the angle is equal to 90° , then decrease while the angle still increases. It means that the wave forces have a maximum value for $\theta = 90^\circ$, and have the same value for the angles of inflow are symmetric to the y -direction (for example $\theta = 45^\circ$ and $\theta = 135^\circ$).

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水流存在時作用於大型結構物之波力解析

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摘 要

本文提出一數值模式，計算波流場中作用於大型結構物之波力。模式中將波流共存場中之速度勢表示為定常水流速度勢及非定常波浪速度勢之組合。由於實際上在結構物周邊水流速度將受結構物存在影響而有所變化，因此水流速度勢應分為穩定均勻流速度勢及水流受結構物影響之擾動速度勢。但當結構物斷面為任意形狀時，水流受結構物影響之擾動速度勢求解不易，本文則應用表面漩渦法(surface vorticity method)，以數值方法直接計算求得水流受結構物影響後之速度變化，而不求解水流受結構物影響之擾動速度勢。同時考慮非定常波浪速度勢為受水流及結構物影響之波浪速度勢，應用繞射理論將波浪速度勢分為入射波及散射波速度勢，其中入射波速度勢係受水流影響後之速度勢為已知值，再利用邊界元素法數值計算波浪受結構物影響後之散射波速度勢，因而結構物周邊之波浪速度勢即可獲得。最後將計算得到之結構物周邊波浪速度勢及水流受結構物影響後之速度變化，代入第一階壓力方程式中，則作用於結構物周邊之波力即可求得。本文以大型圓柱為例，計算不同之水流速度及波流交會角度時，作用於大型圓柱之波力，計算結果與松井徹哉等(1991)之計算例比較，二者之結果尚稱吻合，顯示本文模式適用於波、流與結構物交互作用問題。

關鍵詞：波力、大型結構物、波流共存場。